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DRIFT-PLATTEVILLE AQUIFER

SOURCE AND GRADIENT CONTROL WELLS

CONSTRUCTION AND AQUIFER TEST REPORT

DRIFT-PLATTEVILLE AQUIFER SOURCE AND GRADIENT CONTROL WELLS CONSTRUCTION AND AQUIFER TEST REPORT

Submitted to

U.S. Environmental Protection Agency, Region V and Minnesota Pollution Control Agency

Submitted by

Reilly Tar & Chemical Corporation
. Indianapolis, Indiana

Pursuant to
Remedial Action Plan Section Numbers 9.1.2 and 9.2.2
Exhibit A to the Consent Decree in
United States of America, et al. v. Reilly Tar & Chemical Corp., et al.
U.S. District Court, District of Minnesota, Civil No. 4-80-469

December 9, 1987

INTRODUCTION

Construction and testing have been completed for the Drift-Platteville Aquifer Source and Gradient Control Wells at the Reilly Tar & Chemical Corporation N.P.L. Site in St. Louis Park, Minnesota in accordance with Work Plans submitted by Reilly per the Consent Decree-Remedial Action Plan (RAP). This report presents the logs for three new well installations, the results of pumping tests performed at each new well, and field adjustments to the approved designs for each well. This document fulfills the reporting requirements of RAP Sections 9.1.2 and 9.2.2.

Approvals for the Drift Platteville Aquifer Source Control Well Work Plan and the Drift-Platteville Gradient Control Well Work Plan were received on July 9, 1987. The last permits required for construction were received on August 11, 1987; and, in accordance with RAP Sections 9.1.2 and 9.2.2, all construction, testing, and reporting have been completed within 120 days of that date (December 9, 1987). Construction included the installation of the three wells, construction of appropriate well houses, and connections to the sanitary sewers. Pumping tests included monitoring water levels in the Drift-Platteville Aquifer in numerous observation wells during periods of stressed and non-stressed aquifer conditions.

WELL CONSTRUCTION

The Drift-Platteville Aquifer Source and Gradient Control Wells were drilled and installed in accordance with the approved, amended Work Plans. There were no field adjustments to the approved designs of each well. Bergerson-Caswell, Inc. constructed each well under the direction of ERT, Inc. The Water Well Record for each well is presented in Appendix A, along with the pilot soil boring logs and grain size analyses. As indicated on the Water Well Records, direct rotary techniques were used to advance a nominal 10-inch diameter hole through the drift. Six-inch well-screen and/or pipe was then set in the hole and grouted into place. A graded sand pack was placed around the screens in the two Drift Aquifer wells. The Platteville Aquifer well was completed by drilling a nominal six-inch diameter hole through the full thickness of the Platteville Limestone.

Upon completion of the wells, each well was developed. The Drift Aquifer Source Control Well was developed for approximately 32 hours, using a combination of air-lifting and high-velocity jetting. This well responded nicely to the development by producing a sediment-

free discharge with an approximate specific yield of 10 gpm per foot of drawdown during development.

The Platteville Aquifer Source Control Well was developed by air-lifting. A sediment free discharge was obtained after four hours with an approximately specific yield of only two gpm per foot of drawdown.

The Drift-Platteville Aquifer Gradient control Well was developed by airlifting and high-velocity jetting. A total of 42 hours was spent developing this well. The well was slow to respond to development, although there were no fine grained silts or clays encountered in the pilot soil boring or the well itself that could have accounted for the slow development. At the end of development the well was producing a sediment-free discharge with an approximate specific capacity of 7 gpm per foot of drawdown.

The measuring point elevants for each well were surveyed following completion of each well, with the following results:

Drift Aquifer Source control Well:

Platteville Aquifer Source Control Well:

By 5.83 Feet MSL

Platteville Aquifer Gradient Control Well:

908.18 Feet MSL

DRIFT AQUIFER SOURCE CONTROL WELL AQUIFER TEST

The Drift Aquifer Source Control Well aquifer test was the first of three aquifer tests performed. In preparation for the three tests, potential observation wells in the area were located and examined. Wells that were not sealed, damaged, or destroyed were subjected to a brief slug test to demonstrate the well's response to hydraulic stress. The following wells recovered to 90 percent of their original water level after the removal of one well volume of water, and were considered for use as observation wells during the aquifer tests:

P8	P124	W20
P11	P201	W22
P14	P203	W26
P15	W2	W100
P69	W5	W143
P109	W9	W-1*
P110	W11	W-2*
P112	W17	NL-1*
P121	W18	MW-1*
P123	W 19	MW-2*

These are monitoring wells at the National Lead/Taracorp site.

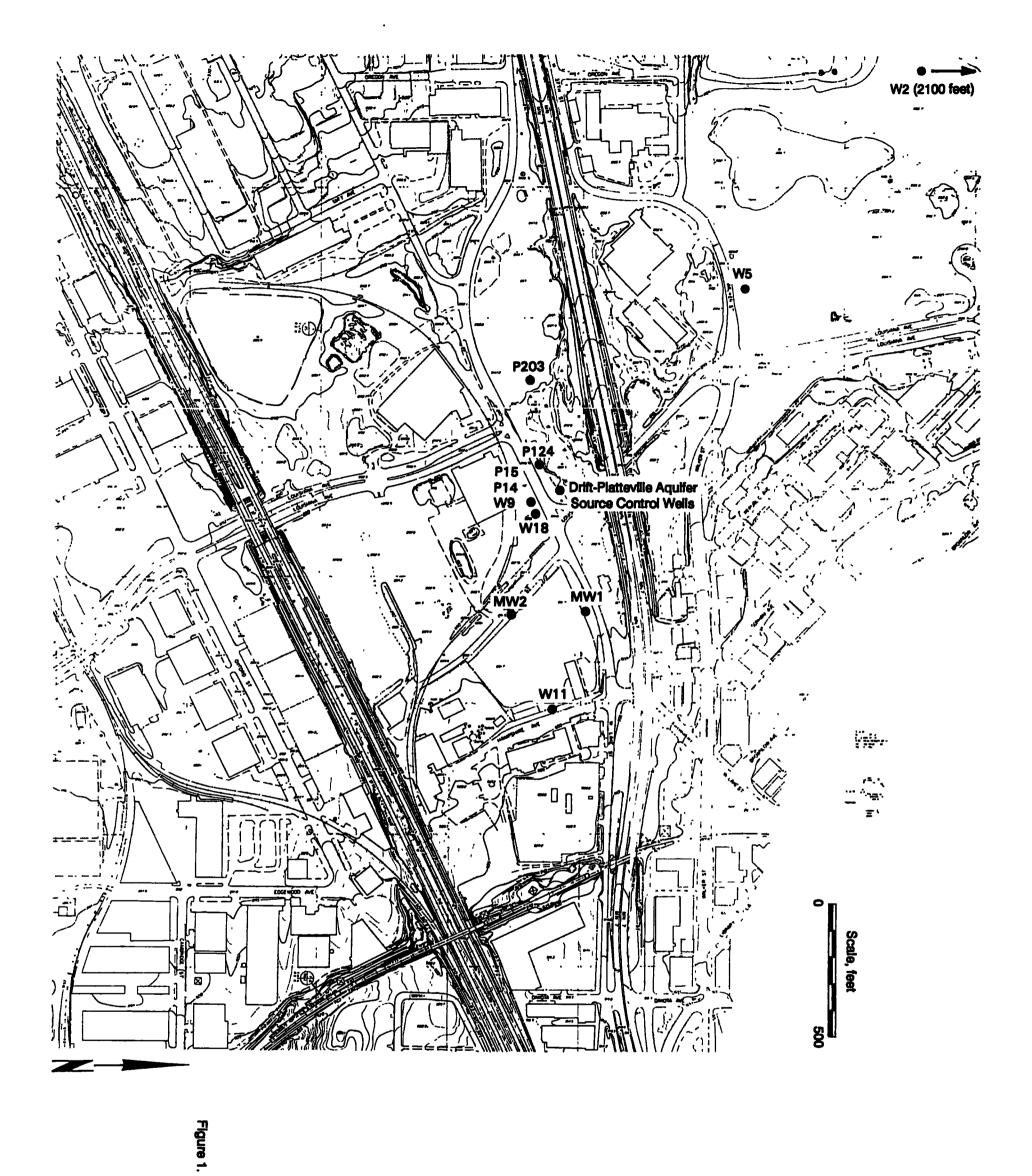
The following wells were not considered for use as observation wells during the aquifer tests because the were either damaged or they did not respond sufficiently during slug tests: W27, W13, W12, W132, P9, P45, P65, P25, P118, and P122.

The observation wells selected for use during the Drift Aquifer Source Control Well aquifer test were wells W2, W5, W9, W11, W18, P14, P15, P124, P203, Taracorp wells MW-1 and MW-2, and the Platteville Aquifer Source Control Well. This array of wells is shown in Figure 1, and includes at least one Drift Aquifer observation well within 200 feet, and one Drift Aquifer observation well within 1000 feet, of the pumping well, as indicated in the Work Plan. Well W2 was used to identify extraneous influences (e.g., water level changes due to precipitation) because it is beyond the cone of influence of the pumping well.

In-Situ, Inc. Hermit data loggers were used to record water level data for wells W2 and W5 for this test, and an In-Situ, Inc. SE-200 system was used for the other 11 wells. Water levels were recorded for 48 hours prior to the pumping phase of the aquifer test. The Drift Aquifer Source Control Well was pumped at 200 gallons per minute commencing on September 17, 1987 at 7:47 p.m., and ending on September 20, 1987 at 10:37 p.m. (approximately 75 hours). Water levels were then measured for 48 hours during which time the levels recovered to pre-test conditions. Data was lost from Taracorp well MW-2 during the pumping phase of the aquifer test due to an interruption in the transducer circuit.

Many of the observation wells exhibited water level fluctuations in response to the aquifer test. Five of the eight observation wells in the Drift formation produced data amenable to analysis (P14, P15, W9, P124, P203). The data from these wells were analyzed using the Prickett method for analysis of pumping test data from an unconfined aquifer (Prickett, 1965). The main assumptions inherent in the Prickett method are as follows:

- aquifer is homogeneous, isotropic, of uniform thickness and has infinite areal extent,
- 2) pumping well fully penetrates unconfined aquifer, implying horizontal flow,
- 3) flow to the pumping well is not steady state and
- 4) well storage is neglected.



 Location of Wells for the Drift Aquifer Source Control Well Aquifer Test Hantush (1964) suggests that if the pumping well is only partially penetrating, drawdown in wells within 1.5 times the thickness of the aquifer may need to be corrected because the assumption of primarily horizontal flow could be violated. The correction is:

s' = s - s²/2d where: s is the original drawdown d is the thickness of the aquifer and s' is the corrected drawdown.

For the first pump test, wells P14, P15 and W9 are within 1.5 times d, assuming d = 60 feet and the radial distance for each of the wells is 75 feet. The maximum drawdown in these three wells was 1.8 feet, producing a corrected drawdown of 1.77 feet. Because the maximum correction is so small, the analyses were done without correcting for partial penetration.

The data and the matchpoints used in the analyses for two representative wells (P15 and P124) are shown in Figures 2 and 3. The values calculated for the transmissivity for the first pump test range from 6.2e+4 gpd/ft to 1.7e+5 gpd/ft (Table 1), with a mean value of 1.0e+5 gpd/ft. These values are similar to these reported in the literature (CDM, 1985; Stark and Hult, 1984; Hult and Schoenburg, 1984; Hickok, 1981; and Barr, 1977). The values calculated for storativity range from 2.5e-2 to 4.5e-1 (Table 1), with a mean value of 1.4e-1. These are typical values for an unconfined aquifer.

The prepumping data for each of the wells analyzed do not show any appreciable trends, although the transducer in well W9 did show some instability with time, both in the background data and the pump test data. For this reason, only the early time data from well W9 were analyzed. The background well data (W2) did not show any appreciable changes over the duration of the first pump test (Figure 4) and were not corrected for in the analyses.

The barometric pressure data and precipitation data (collected from the Minneapolis-St. Paul Airport) over the duration of the three pump tests is presented in Figure 5. The background well data for all three pump tests do not show any response to the variations in these parameters and therefore the pump test data were not corrected for the effects of precipitation or variations in barometric pressure.

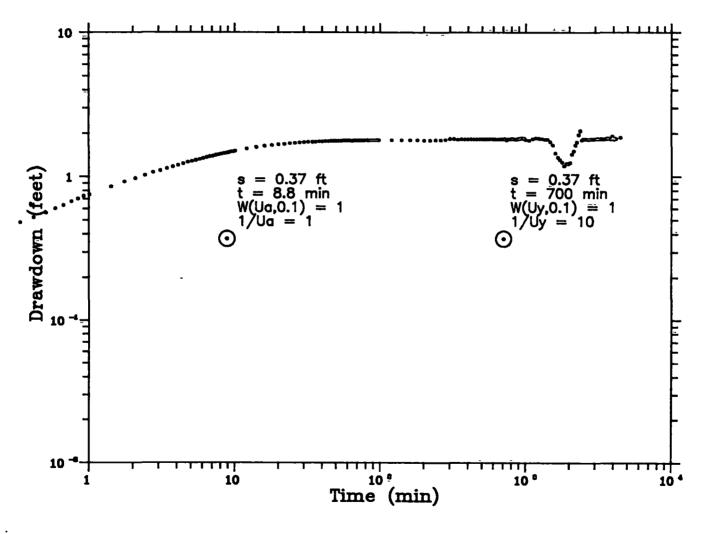


Figure 2. Drift Aquifer Source Control Well Aquifer Test

Drawdown Curve for Observation Well P15

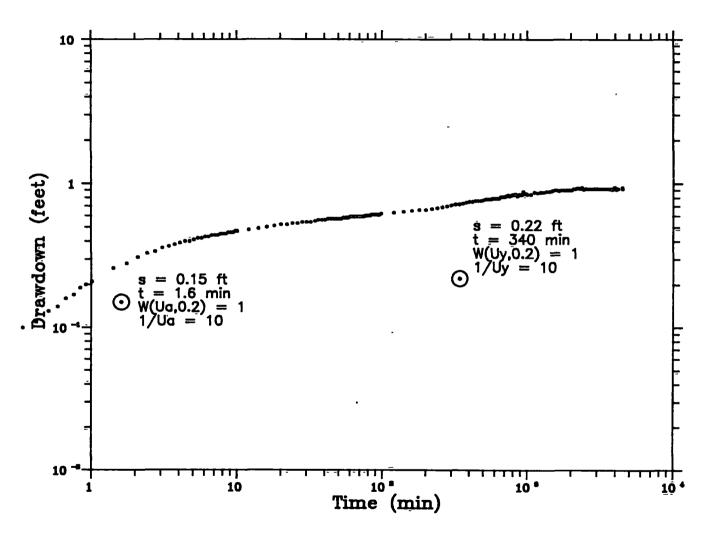


Figure 3. Drift Aquifer Source Control Well Aquifer Test,

Drawdown Curve for Observation Well P124

	,	TABLE 1	
		TRANSMISSIVITY VALUES FROM	тне
	DRIFT A	QUIFER SOURCE CONTROL WELL A	AQUIFER TEST
	WELL NO.	TRANSMISSIVITY (GPD/F	T) STORATIVITY
	P14	7.7'× 10 ⁴	1.47 x 10 ⁻¹
	P15	6.2 ⁻ x10 ⁴	2.86 x 10 ⁻¹
ı.L.)	W 9	7.2 x 10 ⁴	3.83 x 10 ⁻²
	P124	1.3 x 10 ⁵	2.51 x 10 ⁻²
	P203	1.7 x 10 ⁵	1.9 x 10 ⁻¹
		Average $T = 1.0 \times 10$	5 gpd/ft
		Average $S = 1.37 \times 10^{-1}$	_
_			

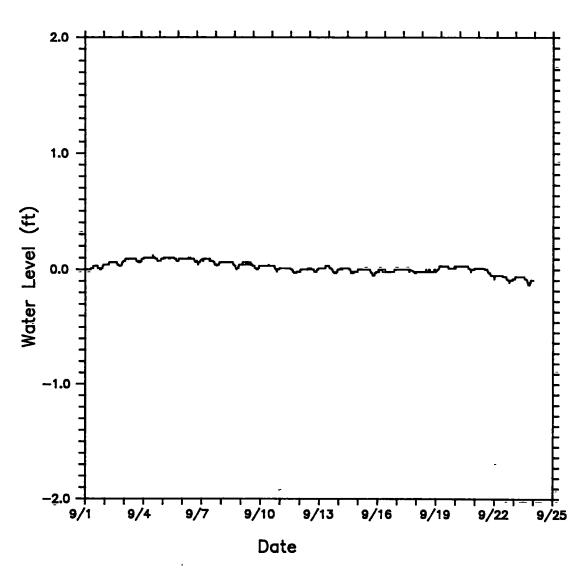


Figure 4: Drift Aquifer Source Control Well Aquifer Test,

Background Water Level Data - Observation Well W2

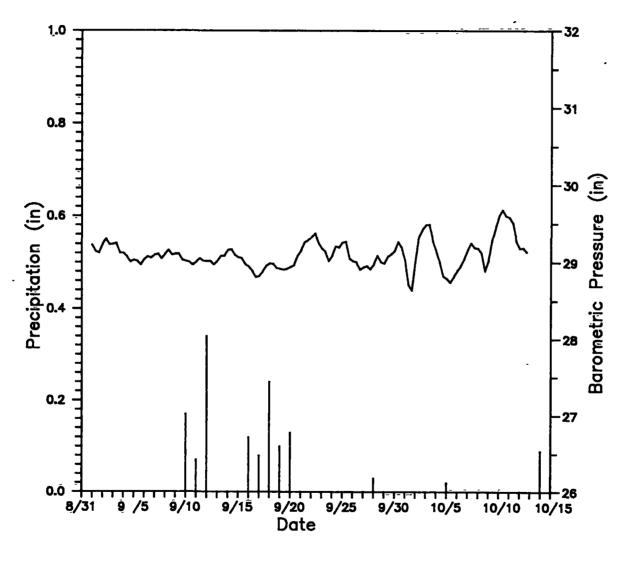


Figure 5. Precipitation and Barometric Data

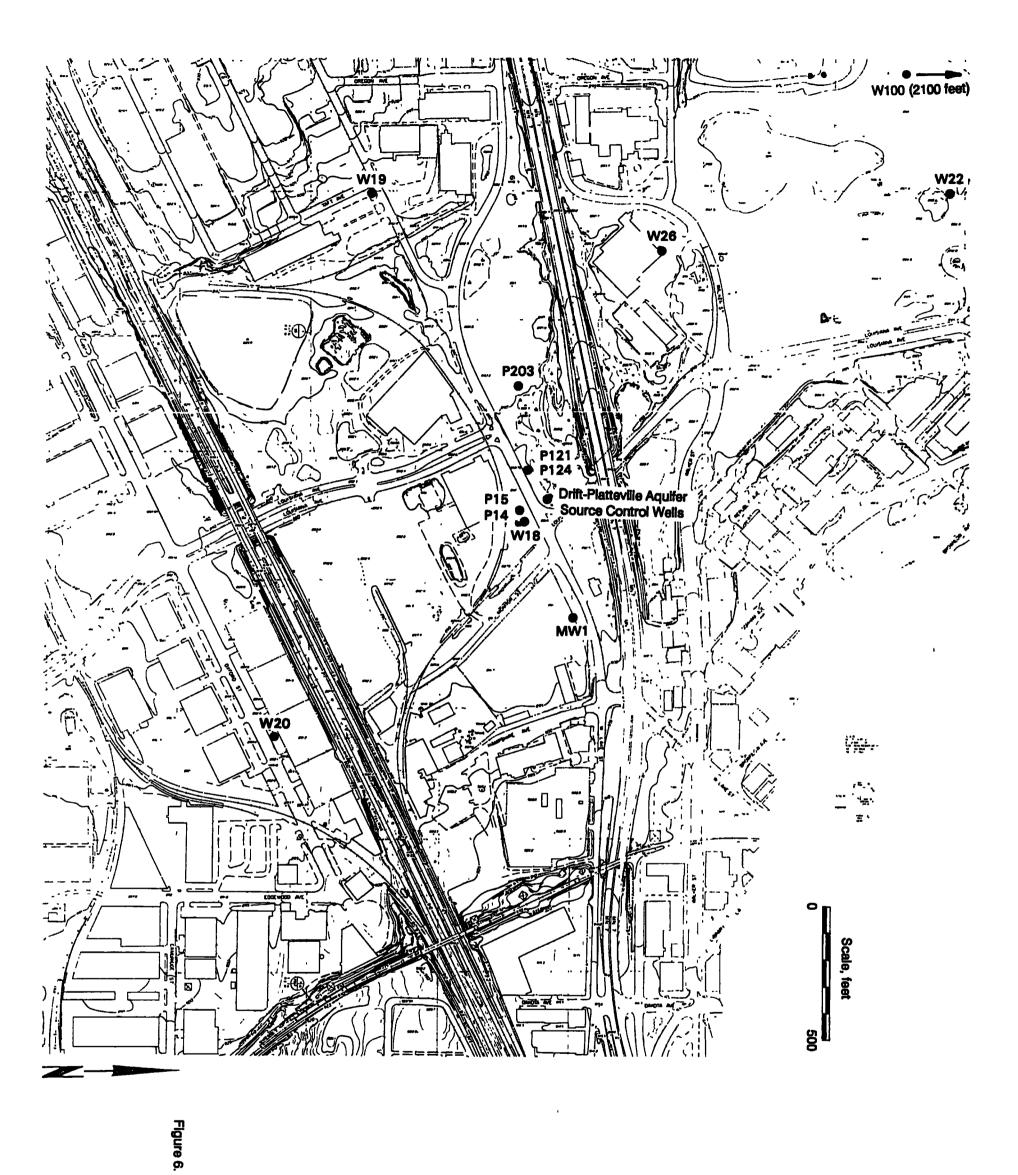
PLATTEVILLE AQUIFER SOURCE CONTROL WELL AQUIFER TEST

The observation well selected for use during the Platteville Aquifer Source Control Well aquifer test were wells W18, W19, W20, W22, W26, W100, P14, P15, P121, P124, P203, Taracorp well MW-1, and the Drift Aquifer Source Control Well. These wells are shown in Figure 6. As indicated in the Work Plan, the array of observation wells for this aquifer test includes at least one Platteville Aquifer observation well within 300 feet, and one Platteville Aquifer observation well at about 1000 feet, from the pumping well. Well W100 was used to identify extraneous influences (e.g., water level changes due to precipitation) because it is beyond the cone of influence of the pumping well.

In-Situ, Inc. Hermit data loggers were used to record water levels in wells W100 and W20, and an In-Situ, Inc. SE-200 system was used for the other dozen wells. Water levels were recorded for 48 hours prior to the pumping phase of the aquifer test. The Platteville Aquifer Source Control Well was pumped at 55 gallons per minute commencing on September 26, 1987 at 10:08 p.m., and ending on September 27, 1987 at 10:20 p.m. (approximately 24 hours). Higher pumping rates caused too much drawdown in the well. Water levels were then measured for 41 hours during which time the levels recovered to pre-test conditions. Data was lost from Well W-22 during the recovery phase of the aquifer test due to an interruption in the transducer circuit.

Many of the observation wells exhibited water level fluctuations in response to the aquifer test. Five of the six Platteville observation wells produced data amenable to analysis (W18, W19, W22, W26, P121). Because the Platteville has been shown to be hydraulically linked with the Drift (Hult and Schoenburg, 1984), the data from the second pump test were analyzed using the Walton method for semi-confined aquifers (Walton, 1962). The main assumptions inherent to the method are as follows:

- 1) aquifer is homogeneous, isotropic, of uniform thickness and infinite areal extent,
- 2) pumping well fully penetrates the semi-confined aquifer,
- 3) overlying the aquitard is an unconfined aquifer,
- 4) water removed from storage is discharged instantaneously with decline of head and,
- 5) well storage is neglected.



Location of Wells for the Platteville Aquifer Source Control Well Aquifer Test

The data matchpoints used in the analyses are shown in Figures 7 and 8 for two representative wells (W18, W19). The values calculated for transmissivity for the second pump test range from 2.0e+4 gpd/ft to 4.4e+4 gpd/ft (Table 2), with a mean transmissivity of 2.8e gpd/ft. These values fall within the range reported in the literature of 2.32e+4 (Hult, 1981) to 6.7e+4 gpd/ft (Stark and Hult, 1984) for the Platteville.

The prepumping data for each of the wells analyzed did not show any appreciable trends. However, the transducers in wells P121 and W26 both exhibited instabilities with increasing time. For this reason, only the early time data from these wells were used in the analyses.

The background well data (W100) did not show any appreciable changes over the period of the second pump test (Figure 9) and were not corrected for in the analyses.

The results from the first two pump tests suggest that the Platteville and Drift aquifers are hydraulically connected. During the Drift source control pump test, several of the observation wells screened in the Platteville showed noticeable drawdown, and wells P14 (Drift) and W18 (Platteville) exhibited responses that are typical of mutually leaky aquifers (Hantush, 1967). During the Platteville source control pump test, the response of the observation wells in the Platteville was typical of a semi-confined aquifer. The observation wells in the Drift did not show as great a response to the pumping of the Platteville, but this is partially due to the difference in pumping rates during the first two tests.

DRIFT-PLATTEVILLE GRADIENT CONTROL WELL AQUIFER TEST

The observation wells selected for use during the Drift-Platteville Aquifer Gradient Control Well aquifer test were wells W2, W11, W17, W143, P109, P110, P112, Taracorp wells W-1, W-2, MW-1, MW-2, and NL2. This array of wells is shown in Figure 10, and it includes at least one Drift Aquifer observation well within 600 feet, and at least one Drift Aquifer observation well within 1000 feet, of the pumping well as required by the Work Plan. Well W2 was used to identify extraneous influences (e.g., water level changes due to precipitation) because it is beyond the cone of influence of the pumping well.

In-Situ, Inc. Hermit data loggers were used to record water levels at wells P109, P110, and P112. The background well (W2) was monitored manually using an electric water level tape throughout the aquifer test. An In-Situ, Inc. SE-200 system was used to monitor water

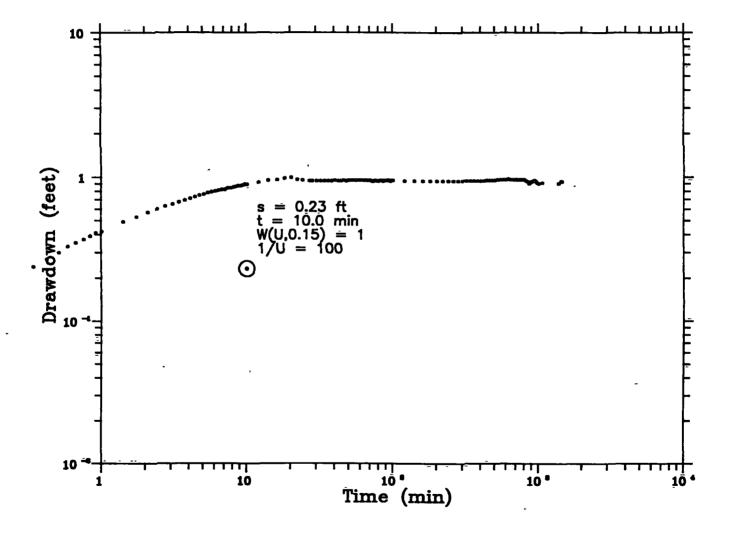


Figure 7. Platteville Aquifer Source Control Well Aquifer Test,

Drawdown Curve for Observation Well W18

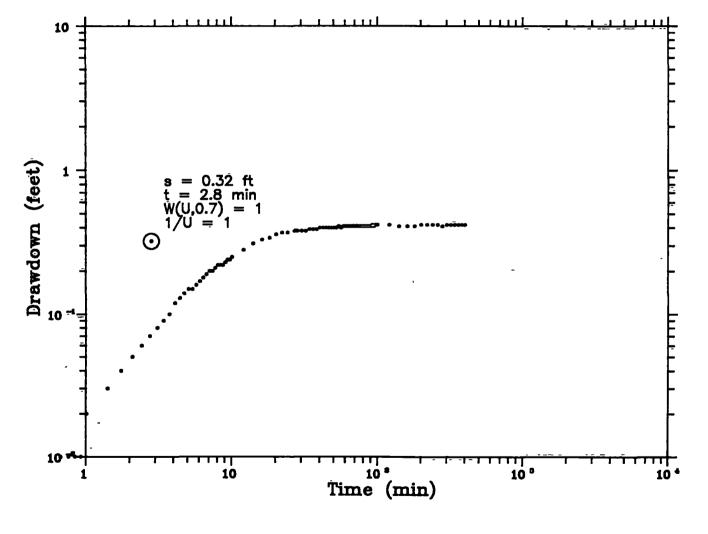


Figure 8. Platteville Aquifer Source Control Well Aquifer Test,

Drawdown Curve for Observation Well W19

TABLE 2

TRANSMISSIVITY VALUES FOR THE PLATTEVILLE AQUIFER SOURCE CONTROL WELL AQUIFER TEST

WELL NO.	TRANSMISSIVITY (GPD/FT)	STORATIVITY
W19	2:0 x 10 ⁴	1.08 x 10 ⁻⁵
W22	4.4 x 10 ⁴	4.89 x 10 ⁻⁵
W18	2.8 x 10 ⁴	1.87 x 10 ⁻⁴
P121	2.5 x 10 ⁴	1.8 x 10 ⁻⁵
W26	2.4 x 10 ⁴	8.8 x 10 ⁻⁵

Average $T = 2.8 \times 10^4 \text{ gpd/ft}$

Average S = 7.05×10^{-5}

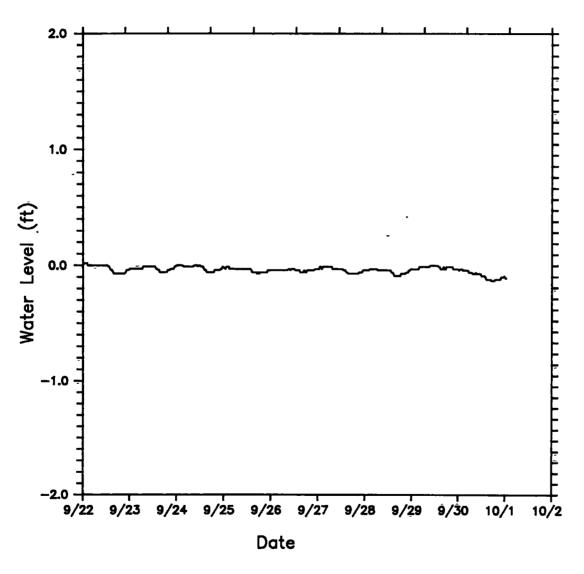


Figure 9. Platteville Aquifer Source Control Well Aquifer Test,

Background Water Level Data - Observation Well W100

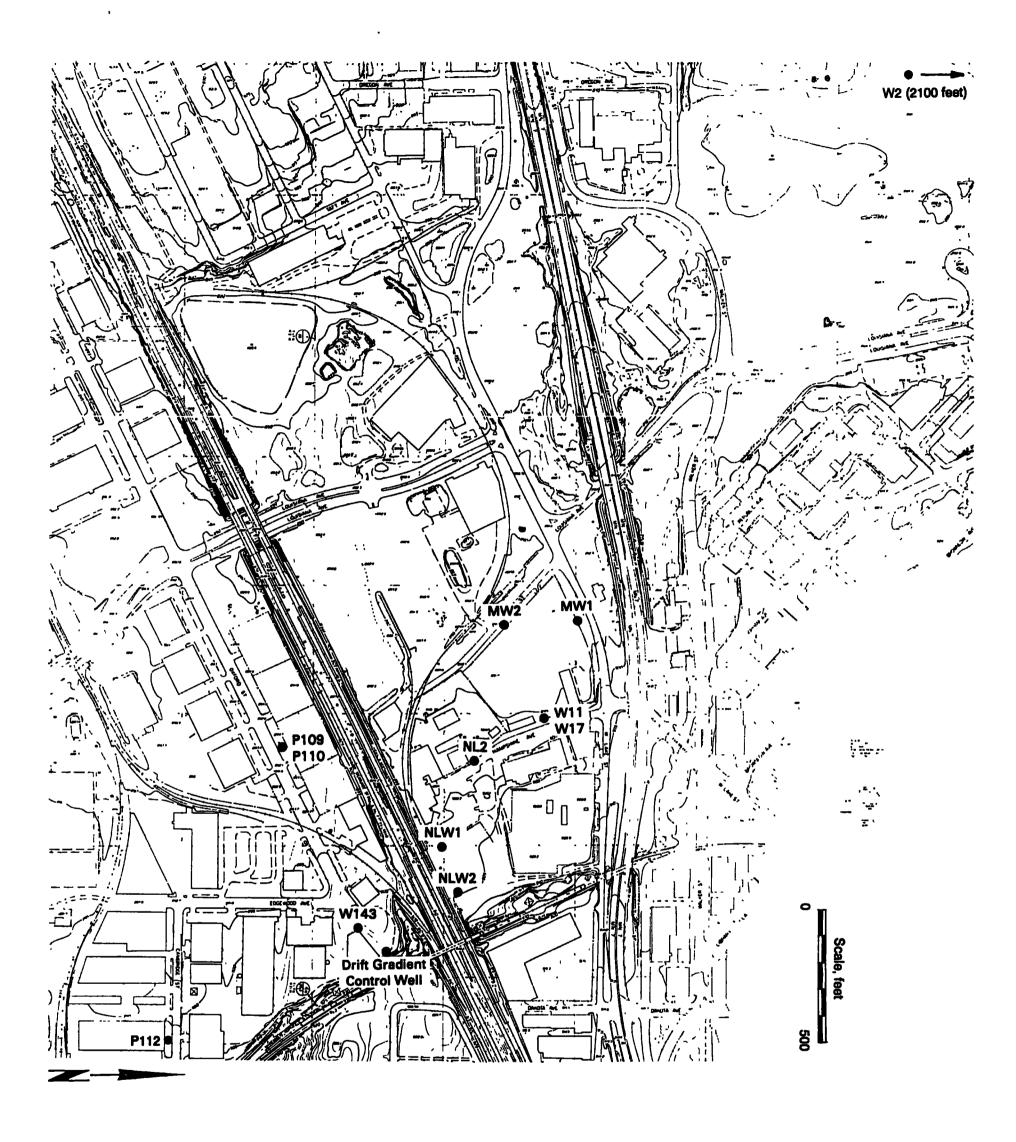


Figure 10. Location of Wells for the Drift Aquifer Gradient Control Well Aquifer Test

levels at the remaining wells. Water levels were recorded for 48 hours prior to the pumping phase of the aquifer test. The Drift-Platteville Aquifer Gradient Control Well was pumped at 90 gallons per minute commencing on October 4, 1987 at 3:40 p.m., and ending on October 8, 1987 at 2:20 p.m. (approximately 95 hours). Higher pumping rates caused too much drawdown in the well. Water levels were then measured for 48 hours to allow for recovery. Data was lost from well W143 at the start of the aquifer test because of an interruption in the transducer circuit.

This aquifer test was different from the first two tests because most of the observation wells did not exhibit water level fluctuations in response to the aquifer test. However, one of the ten observations wells for the third pump test produced data amenable to analysis (W17). The data from this well were analyzed using the Prickett method, with the assumptions presented earlier.

The data and matchpoints used in this analysis are shown in Figure 11. The value calculated for the transmissivity is 5.9e+4 gpd/ft which is within the range of literature values presented for the Drift aquifer and in fair agreement with the results of the Drift source control well pump test above. The value calculated for storativity is 1.06 x 10⁻²

The prepumping data for well W17 did not show any appreciable trends and were not corrected for in the analysis. The background well data (W2) did not show any measurable changes over the duration of the third pump test (Figure 12) and were not corrected in the analysis.

The lack of analyzable data from many of the observation wells for the third pump test is in part due to the lower pumping rate for the pump test (90 gpm), the lack of nearby observation wells that were screened at the same depth as the pumping well, and a lower transmissivity for this portion of the Drift aquifer.

SUMMARY

The Drift-Platteville Aquifer Source and Gradient Control Wells have been installed and tested in accordance with approved plans and schedules and Sections 9.1.2 and 9.2.2. of the RAP. Raw data generated during the three aquifer tests have been transmitted to the Minnesota Pollution Control Agency (Schaefer, 1987). In addition, the data are available for inspection and review at ERT, Inc.'s office in Concord, Massachusetts. Well houses have been

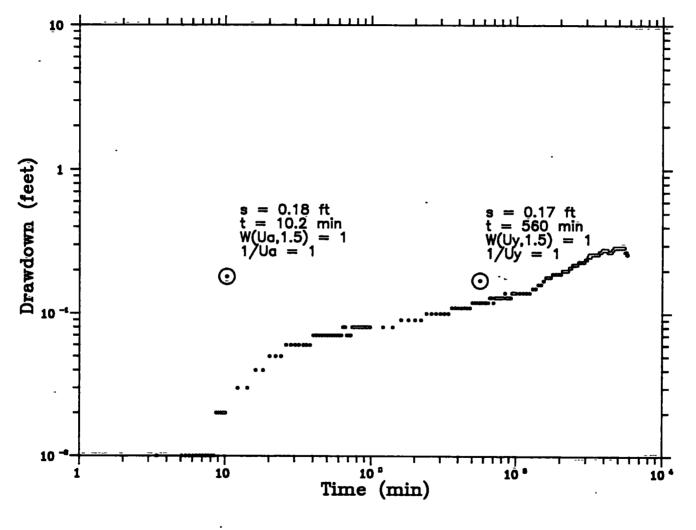


Figure 11. Drift-Platteville Aquifer Gradient Control Well Aquifer Test,

Drawdown Curve for Observation Well W17

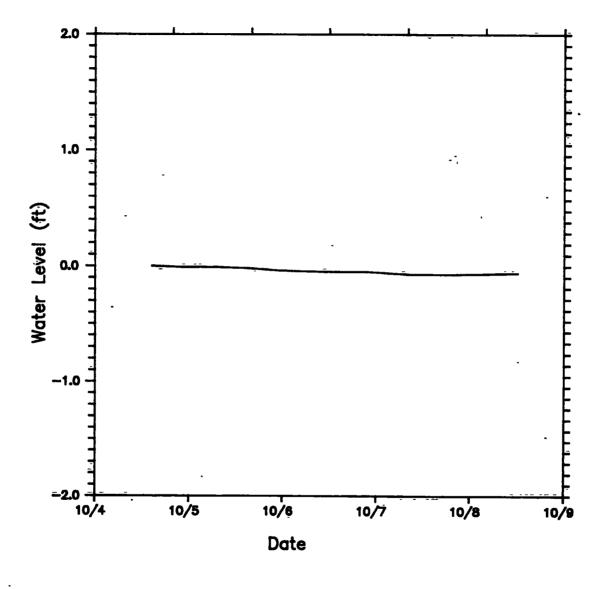


Figure 12. Drift-Platteville Gradient Control Well Aquifer Test,
Background Water Level Data - Observation Well W2

	constructed and connections with the sanitary sewer system have been made for the discharge
	from the pumping wells. The three wells are now ready to commence operation in accordance with the requirements of the RAP.
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REFERENCES Barr, 1977. Soil and Groundwater Investigation, Coal Tar Distillation and Wood Preserving Site, St. Louis Park, Minnesota: Report to the Minnesota Pollution Control Agency. Barr Engineering, June 1977. Camp, Dresser and McKee. 1985. Phase 1 Final Report, New Brighton/Arden Hills. Minnesota Multi-Point Source Remedial Investigation, prepared for Minnesota Pollution Control Agency. Hantush, M.S. 1964. Hydraulics of Wells, in: V.T. Chow (ed) Advances in Hydrosciences, vol. 1:281-432. Academic Press, New York and London. Hantush, M.S. 1967. Flow to Wells in Aquifers Separated by a Semi-Pervious Layer. Journal of Geophysical Research, vol. 72, no. 6. Hickok, 1981. Study of Groundwater Contamination in St. Louis Park, Minnesota: Submitted to the Minnesota Department of Health. Eugene A. Hickok and Associates (Wayzata, MN); Geraghaty & Miller, Inc., Henningson, Durham & Richardson, Inc., November 1981. Hult, M.F. 1981. Letter to J. Erdman, E.A.Hickok & Associates. August 3, 1981. Hult, M.F. and Michael E. Schoenburg. 1984. Preliminary Evaluation of Ground-Water Contamination by Coal-Tar Derivatives, St. Louis Park, Minnesota. U.S. Geological Survey Water Supply Paper 2211, 1984. Prickett, T.A. 1965. Type-Curve Solution to Aquifer tests under Water-Table Conditions. Groundwater, vol. 3, no. 3:5-14. Schaefer, M. 1987. Letter to Justin Blum, Minnesota Pollution Control Agency. October 23, 1987. Stark J.R. and M.F. Hult, 1984. Ground-water-flow model of the Prairie Du Chien-Jordan aquifer, St. Louis, Park, Minnesota, U.S. Geological Survey-Water Resources Investigation Report 84-In Review. Walton, W.C. 1962. Selected Analytical Methods for Well and Aquifer Evaluation. Illinois State Water Survey Bulletin, no. 49, 81p.

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	Drift-Platteviile Aquifer Source and Gradient Control
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J	Grain Size Analyses
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DRIFT AQUIFER SOURCE CONTROL WELL	
o Water Well Record	•
o Pilot Soil Boring Log	
o Grain Size Analyses	
·	

	County Name				VELL REC	/ Water Care 1
П	Hennepin Township Name! Township Nu	mber Range Number S			itales 156A	4 WELL DEPTH (completed) [/Date of Completion
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LJ						14. NEAREST SOURCES OF POSSIBLE CONTAMINATION
п		 				feettype
Ш			·			Well disinfected upon completion? C) Yes Cl No
ا ہے						Date installed 10/12/87 Date installed
						Manufacturer's name Grundfos Model number SP6-10 HP 2 Volta 200
ш						Model number SP6-10 HP 2 Volta 200 Length of drop pipe 42 ft. capacity 25 g p.m.
						Material of drop pipe Steel
			<u> </u>			Type: 15 Submersible 313 L.S. Turbine 513 Reciprociting
		1	1		ř 1	20 Jet 40 Centrifugal 60
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	,					Name of Oriller
Ų.	IMPORTAN		. 43	340	45 l	8/74 30M 7/8 30M 7/8 30M

Soil Boring Log, Drift Aquifer Source Control Well ARCHITECT-ENGINEER PROJECT NAME Proposed Well House **ERT** STS Consultants Ltd. SITE LOCATION UNCONFINED COMPRESSIVE STRENGTH TONS-FT ' St. Louis Park, Minnesota 3 WATER CONTENT % EQUID LIMIT % PLASTIC feet SAMPLE DISTANCE **×** ∽ DESCRIPTION OF MATERIAL UNIT DRY WT. LBS /FT * оєртн і п SAMPLE TYPE ELEVATION 10 30 40 50 SAMPLE NO RÉCOVERY STANDARD PENETRATION BLOWS FT SURFACE ELEVATION 20 30 40 50 Silty fine to medium sand, trace ⊗5 SS 1 coarse sand and roots - dark brown to black - moist - loose - (SM) . 2 SS **X**12 Fine to coarse sand, little gravel 9 little silt - brown - wet to saturated - medium dense - (SM) -**⊗**4 SS **3A** Fine fibrous peat - black - (Pt) **Ø**5 SS 10.0 68 5 SS Amorphous peat - black - (Pt) 6 SS 11 16 6A SS <u>15.0 6</u> $\otimes 4$ Organic silty clay, trace fibers white - firm - (OH) 20.0 |/7 SS,111== 7A SS CR 10 Fine to coarse sand, trace gravel and silt - gray - saturated mediúm dense - (SP) Note: Cresote like odor in sample 25.0 7 and 8. (X) 10 8 SS End of boring at 26.0 ft. calibrated pehetrometer Hollow stem auger to full depth. THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES IN SITU, THE TRANSITION MAY BE GRADUAL WL WS OR WD STS OFFICE **BORING STARTED** 2.3 ft. WS 9/29/86 <u>Minnesota</u> 9/29/86 SHEET NO. OF KC WL **BCR** ACR **BORING COMPLETED** DRAWN BY

CME-45 FOREMAN

APP'D BY

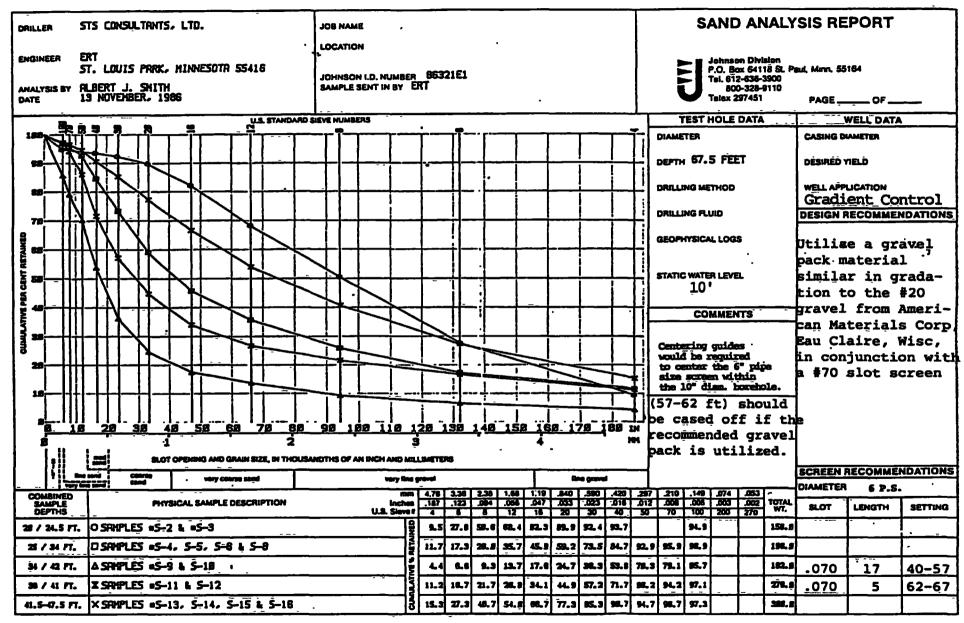
WBT

STS JOB NO.

94026

1.5 ft. AB

G	7	•													
ST3 Co	Leuil	Lamb		1	PROJECT NAME Proposed Vall Hou	fa		ARCHITE	CT—EN	ginee	A				
SITE L					St. Louis Park, H	Inneseta				O 5		•	M (The		
DEPTIV	SAUPLE NO.	SAUNE TYPE	MPLE DISTANCE	ECOVERY		DESCRIPTION OF MAI	TERIAL.		UNIT ORY WT.	1 -	4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				
X	3	3	Z	2	SURFACE ELEVATION + Hollow stem augere	100 feet	ana lan	of booles de	13-	ļ		20	10	10	9
			1		for soil condition	s above 17.5 fee	t	-	ļ,			1			
13.2		22			Organic slity clay (OH)	, little sand -	dark brow	n + firm -		6	3 7	4-			
	2 2A	55	Ш		Kinder and Brok) 10	Ī	 		
	Ŀ	55		Ξ	Silty sand - gray	- saturated - me	dium dens	a - (SM)	ــــــــــــــــــــــــــــــــــــــ		224.	<u> </u>			<u> </u>
25.0	5	32			Fine to coarse san saturated - loose Note: 31.0 feet d					8	9 6 9 10				
	7	55	Н		Fine to medium gravel, trace sile (SP)	sand, trace coar t - brown - satu	se send, reted - m	trace edium dense -	\		8.		930		
35.3	9	55			Silty sand, trace (dense - (SM)				+			8	27		
	10A	55	-	-	Sandy gravel, trace dense - (GP)	silt - gray -	sacurated	- medium	+	8	, e	112	14/6.		
	12 13 14 15 16 17 18 19 20 21 22	\$\$ \$\$ \$\$ \$\$ \$\$ \$\$ \$\$			Fine to coarse sand saturated - medium	. little silt, 1	<u></u>	lt - gray -		8	* 2/ 5/8/8/ W\@\&	© 23 ⊗ 22 9:9	27	9	
	23	┝━		4	medium dente - (SH)					 	,, Q	1		22/6	
94.3	24 25 (55 55	H	П	Sandy gravel, trace dense - (GP)	======================================	isturated	- medium		ļ	24 21	L/⁻			
	26	5.5		П	fine to coarse sand gray - saturated -	, trace to littl	e gravel, SV) - (SP	trace silt -	1	-	190				
\$	27 28	55 55			Sandy gravel, tra dense - limestone	co silt - grav -		d a midle	4	100	7	3 —			ERK.
50.3	29 294	经	Ħ		Clateville linesto		,, 100		- 	=	30	-	 		locy i
					End of boring at 66 Hollow stem sugere 17.5 fc. to 32.5 ft rockbit and benton	.8 ft. d to 17.5 ft. Dr . Caving gravel	illed wit	h 3 15/16" ro Set hollow	cktit ar	d ban	on i te		ing (tuid 2 15	V16"
		<u>_</u>		<u>.</u>	MATTPICATION UNITS REPRESEI						===			4.040	
·WL	_				WS OR WO	SORING STARTED		10/24/86	STS OFFICE	-	Innas		نووي		
W		_	_	_	SCR . ACR	BORING COMPLETED)	10/24/86	DRAWN BY			HEET NO	1	OF 1	!
w		_		_		RIG CHE-75	POREMAN	GD GD	APP'D DY	130	81	18 408 P	na. 94	026	



DRILLER ENGINEER	JOB NAME			·	-			_							YSIS REPORT		
ANALYSIS BY DATE 13 NOVEMBER, 1986	JOHNSON I.D NUMBER SAMPLE SENT IN BY	9632: ERT	E						Johnson Division P.O. Box 64118 St. Paul, Minn. 55164 Tel. 612-636-3900 800-328-9110 Telex 297451 PAGEOF								
U.S. STANDARD S	EVE NUMBERS									7	EST	OLE	DATA			WELL DATA	
			T-T	T				Ī		DIAME	TER				CASING D	MACTER	
		<u> </u>	╬╌	· · ·		· ·		<u> </u>	IJ.	DEPTH	1				DESIRED	AIĒĪ'D	
	- 	!	<u> </u>				<u> </u>			DAILLI	NG ME	THOD			WELL APP	LICATION	
		į į	<u> </u>						Lil ,	DRILLI	NG FLI	лo			DESIGN I	RECOMME	NDATIONS
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<u> </u>								Щ	Ш				•				
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<u>9 18 28 30 48 50 88 78 88</u>	98 198 118 1	<u> 28 13</u>	0 142	15	<u>1 18</u>	8 17		8Ø 11	M						<u> </u>		
SLOT OPENING AND GRAIN SIZE, IN THOUSA		r I ma teria	•		•				"						[ł
					_			_	4						SCREEN	RECOMME	NDATIONS
And the step in th	very fine g			1	1		grand	400 T	丄		0.46 ·1				DIAMETER		
COMBINED SAMPLE DEPTHS PHYSICAL SAMPLE DESCRIPTION	U.S. Sleve#		23 804 6 8	.066 12	047	.003 .003 .20	.023	.D16]	012	.008 70_	200	.88	.002	TOTAL WI.	SLOT	LENGTH	SETTING
47 - 57 pr. ÖSRAPLÉS *\$-18.5-17.5-19.5-28.5-21.5-	-22	┝╌┼╴	1.7 2.0	2.0	6.9	_	48.6	74.8	ķ	98.5				348,6			
57 - 60 FT. DSRIPLES -S-23 & S-24	LATIVE & RETAI	- 	24 12.7	+	20.1	-	23.5	34.5	6.9	54.9	Q1.5			244.6			
ш - се гг. △SRIPLES -S-25, S-28, S-27 & S-28		2.9	7.7 19.2	21.4	31.4	44.4	52.4	60. 4	20. 3	80.5	94.5			234,1			
es - et.s rt. I SREPLES •S-29	- IV	25.5	9.7	62.6	66. 2	71.6	72.4	22.4	65. 1	87.8	99.5			140.5			
	[8]	يلط														<u> </u>	L

SO MANY CONSIDERATIONS ENTER WTO THE MAKING OF A GOOD WELL THAT, WHILE WE SELIEVE SLOT SIZES FURNISHED OR RECOMMENDED FROM SAND SAMPLES AND CORRECT WE ASSUME NO RESPONSIBILITY FOR THE SUCCESSFUL OPERATION OF JOHNSON WELL SCREEKS.

	PLATIEVILLE AQUIFER SOURCE CONTROL WELL	
	o Water Well Record	
<u>-</u>		
	•	
	•	

	County Name			WATER V	NELL REC	ORD MINNESOTA-UNIQUE WELL NO	434044
\Box		p Number Range Number S		Vinnesota St	etules 156A	01 .08 4 WELL DEPTH (completed)	Dute of Completion
U .	St. Louis Park 11 Distance and Direction from Road Intersection or	7 4 21	17	9C1104	4 4	•	10/12/87
П	NV Quad Lake Street & Lou	-	KI Location				3 Driven 1013 Dug
	Show exact location of well the section grad with "		Sketc	h map of we	l location	2C) Hollow rod SC) Air 8C	3 Bored
	N	Addition Name				ł ",	3 Power Muger
\cap		ļ				6. DRILLING FLUID	
U	▎▕ <mark>ੑੑੑ</mark> ╟╌┊╌┤╌┊╼┾╶┧╌┟╶┽╌╏ _╸	Block Number			•	Bentonite 7 USE	
		Lot: Number				ID Domestic 👸 Monitoring	8C) Heat-Pump
П	*"	LOC: IN MANDET				20 tregation 50 Public 30 Test Well 60 Municipal	90 Industry 100 Commercial
Ш			•			8 CASING	110
_ '	2 PRUPERTY OWNER S NAME	City of St.	Louis Pa	rk		Manual HEIGHT A	
		5005 Minneto				Y Surface	
u	Address	St. Louis Pa	•	55416		3C) Plastic 6C)	Yes No.X
п :		Attn: Jim G		,,	,	12 in to 20 ft. Weight —	49.56 lbs/k 12.in to 20.ft
11	3 FORMATION LOG	COLOR	HARDNESS OF FURMATION	FROM	TO	4	18.97 Ba./IL 12,m. to 67 ft
<u> </u>	Sand & Top Soil	Black	S	0	2	9 SCREEN Or	
п	can a rop corr	- Didde		 		Make from	open bale m_ 67 ft ta <u>84</u> ft
	Sand, T.Gravel & Silt	Brown	VS	2	5	Туре	Dis
	n .	701	700		,,	Slot/Gause	LengthFITTINGS
П	Peat	Black	VS	5	15	Set between ft. and ft.	
U	Silty Clay - Organic	White	s	15	20	10 STATIC WATER LEVEL	ate Measured 8/25/87
						land surface 11 PUMPING LEVEL (below tand surface)	Ae Measured
\prod	Coarse Sand & Cravel	Brown	M	20	35		
U						ft after hrs. pumpin	= = = = = = = = = = = = = = = = = = = =
_	Coarse Gravel	Multi	M	35	67	12 HEAD WELL COMPLETION	
	Mad-11- 12		VH	67	84	ICI Pitiess adapter manufacturer	model
w	Plateville Limestone	Orange-Grn	Vn.	0/	- 04	4C) Plastic casing protection	
п						11 WELL GROUTED? XD Yes D No	
						IN Neat Cement 20 Bentonite 3D	
•	·			ļ		Grout material 376 from 0	to 67 ft cu yds 1.0
\prod	1 1	1.					
U	· · · · · · · · · · · · · · · · · · ·					14 NEAREST SOURCES OF POSSIBLE CONTAMIN	ATION
_						feetdirection	
						Well drainfected upon completion? 🗆 Yes 💢 No	
ч	 			 	$\vdash \vdash \vdash$	15 PUMP Date installed 10/12/87	
\neg						Date installed	
						Model number SP6-10	MP 2 Volta 200
_	· 			<u> </u>		Length of drop pape 42 ft.	capacity 25 g p.m.
П	•					Material of drop pipe Steel	
Ш				-			proceling
ı						-20 Jet 40 Centriligal 60	
П						Unused well on property? Yes No	
ן נוי	Un a 1 17 REMARKS, ELEVATION, SOURCE OF DATA	acond short, if medad		نـــــا	L{	Abandoned D Permanent C Temporary C	Not sealed
~	87-6				ſ	· 18 WATER WELL CONTRACTORS CERTIFICATIO	
	Wellhouse finished f	loor elevatio		_	ļ	This well was drilled under my jurisdiction and this r knowledge and belief.	sport as true to the best of any
Ų	Corection to MSL	-	=710.3	10	İ	Bergerson-CAswell Inc.	27058
	Stick up above floor Measuring point elev		=1.56	, o		Licensee Business Name	Lucense No
	(top of well sea	1	=895.7	7		Address 5115 Industrial Street	t Maple Plain, M
	/-25 0- 407T 90G	-,			l	Signed C & House	Date 10/12/87
П					I	Authorised Ripresent	Date 10/12/87
U	IMPORT		T A	340	1 1	Name of Driller	5/14 30M 7/76 30M
	FILE WITH DEED W	ELL OWNER COR	v 14	34U	441	}	7/78 104

DRIFT AQUIFER GRADIENT CONTROL WELL

- o Water Well Record
- o Pilot Soil Boring Log
- o Grain Size Analyses

County Name	 1		WATER I	WELL REC	CORD MINNESOTA UNIQUE WELL NU 434043
Township Name! Township N	umber Range Number			steks ISGA	4. WELL DEPTH (completed) Date of Completion
St. Louis Park 117 Distance and Direction from Road Intersection or Str	劉 21 基	17	¥	4 4	· · · · · · · · · · · · · · · · · · ·
6425 Oxford Street - In Fron		en mari			3CI Cable tool 4CI Reverse 7CI Driven 10CI Dug
Show exact location of well in section grid with "X."	it on city now	Sketch	map of we	I location	2C) Hollow-red 5C) Air 8C) Bored 11C)
	Istion Name				JZS Rotary 6Cl Jetted 9Cl Power Jauger -
	Ţ				6 DRILLING FLUID
] w	ck Number				Bentonite
1 "Landada (1)					1C) Domestic 4D Monstoring 8C) Heat-Pump
	Number				20 Irrigation SCI Public 9CI Industry 3CI Test Well 6CI Municipal 1UCI Commercial
┨ <u>┣</u> ╬═┢┪╫╗┩╬┩╏	· ·			•	70 Air Conditioning 110
154					8 CASING HOLE DIAM
	ity of St. Lou				NO Black 4□ Thresded Surface 2.5 ft
•	005 Minnetonka		,	• ,	, 20 Galv SCA Welded Drive Shoel Yes. No.X
L .	t. Louis Park,		6	,	3D Plastic 6D 12 in to 20 ft. Weight 49 t 56 lbs /ft 12 n to 20 ft.
3 FORMATION LOG	ttn: Jim Grub			1	6 m, to 53 ft. Weight 18, 97 the /tt 12n to 28.
5 FORMATION LOG	COLOR	HARDNESS OF FORMATION	FROM	TO	m. to ft. Weight lba/ft in. to
Sand	Brown	s	0	15	9 SCREEN Or open hole
	- Drown	 	 	1	Make Johnson from fi to fi
Clayey Sand	Brown	l M	15	25	Type Wire Wound Drs 6" PaSa
<u> </u>		ļ		 _	Stor/Gause No. 40
Silty F. Sand; T. Gravel	Brown	M	25	55	Seither ween 53 ft and 78 ft
					10 STATIC WATER LEVEL
Med. Gravel; T. Sand	Multi	<u>H</u> .	55	78	25 It Delow & above Date Measured 9/10/87
	ļ.				land surface 11 PUMPING LEVEL (below land surface)
`			ļ	L	42 ft. after 72 hrs pumping 90 g p m.
	Ì	1]	ft after hrs. pumping g p m
	- 	 	 -		12. HEAD WELL: COMPLETION 1D Puless adapter, manufacturer
i ·	,			İ	2□ Basement offset 3€At least 12" above ground
		 			4D Plastic casing protection
		1			I 13. WELL GROUTED? Li Yes D No
					V V V IO Neal Cerment 20 Bentomile 30
		<u> </u>			Grout material 37 from 0 to 48 ft cu yes 0.5
		1			
	 		<u> </u>		
i			·] ,	14 NEAREST SOURCES OF POSSIBLE CONTAMINATION
		<u> </u>		 -	We'll distrifected upon completion? Cityes & No
	<u> </u>	<u> </u>		<u>i</u>	15 PUMP
		1			Date matalled 10/12/87 O Not installed
		 	ļ		Manufacturer's name Grundfos
					Model number SP16-5 HP 5 Volta 200
			 		Length of drop pipe 42 ft. capacity 50 g.p.m.
	-		·		waters of all by
	 	 			Type 10 Submersible 30 LS Turbine 50 Reciprociting
4			ŀ		20 Jet 40 Centrifugal 60
					IA. EXISTING WELLS • Unused well on property? CI Yes. Xo No.
	ed short, of worded	1	L.,_,	<u>L</u>	Abandoned D Permanent D Temporary D Not sealed
17 REMARKS, ELEVATION, SOURCE OF DATA, et 87 -6		•			· IS WATER WELL CONTRACTORS CERTIFICATION
1				,	This well was drilled under my jurisdiction and this report is true to the best of my
Wellhouse finished fl	oor elevati			ł	knowledge and belief.
Correction to MSL		=710		- 1	Bergerson-Caswell Inc. 27058
Stickup above floor		=1.5			License Bounes Name License No. License No. License No. License No. Street, Maple Plain, MN
Measuring point eleva		=908	3.18		Address Distriction Screen, rapid Fram, ray
(top of well seal	,				Signed C Date 10/12/87 Authorized Representative
1				1	Glenn Holman Page 10/12/87
IMPORTAL	NT:	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	2 / 0	42	Name of Emiles
FILE WITH DEED - WEL		y 43	340	45	9/74 50% 7/76 30% 17 / 78 50%

G	3		۲-			Soil	Boring	Log, Dr					Cont	rol 1	Well 	
STS Con	sultan	te Ltd.	- 1		JECT NAME Oposed We	i i Hou	ıse			HITECT: RT	E NGINI	EER-				
SITE LOC	ATIO	N		St	. Louis Pa	ark, h	linnesot	:a				UNCONFI TONS-FT		PRESSIVI	E STRENG	M S
DEPTH IN FEET ELEVATION	NO.	Three	SAMPLE DISTANCE	ERY		DESCRI	PTION OF M	ATERIAL		UNIT DRY WT. LBS./FT.		LASTIC POINTS	20	WATER NTENT 9		JOURD JAMES &
X DEPT	SAMPLE NO.	SAMPLE TYPE	SAMPLE	RECOVERY	SURFACE ELE	/ATION				OFFIS ABL	. '		NOARD ETRATION	_ N 30	BLQWS	
	1	ss		II.	Fine to r	vel -	brown	little - moist	silt,			Ĭ.				50/1
	2	SS			medium de			trace c	/ parse					<u></u> ⊗	35	
5.0	3	SS			sand and brown - c	grave	i, trac	e silt	- ,				\$20	'		
	4	ss		I	Fine sand	OWI -	ce silt moist	and medium	i lum n			· / Ø	5	1		
1.0	5	ss		П	dense -	(SP)		•				⊗ _I ,	1	<u> </u>	ļ.	
		٠			End of bo Power aug	ered	at 11.0 to full	ft. depth.	•							
									•							
			Ì							,		1				
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H	IE STR	ATIFIC	ATIO	N/UI	YES REPRESENT T	HE APPRO	NUOB STAMIX	IOARY LINES:	ETWEEN SC	L.TYPES.	IN SITU,	THE TRAI	(SITION:N	AAY BE C	RADUAL	<u></u>
L Dr					WS OR WD	BORING	STARTED	9/2	9/86	ts offi	CE		innes	ota		
/L		В	CR		ACR	BORING	COMPLETE	FOREMAN	77 00	PPO'BY			ON TBBI	<u>-</u>	of 94026	1

ARCHITECT-ENGINEER PROJECT NAME Proposed Well House ER, Inc. St. Louis Park, Mirmesota SAMPLE NO.
SAMPLE TYPE
SAMPLE DISTANCE
RECOVERY
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RECOVERY DESCRIPTION OF MATERIAL LES. /FT Hollow stam augured to 14.0 feet — see log of boring 81 for soil conditions above 14.0 feet Fine to medium sand, trace coarse sand and silt - brown - moist - medium dense - (SP) Clayey sand, some silt, trace gravel - brown - moist - medium dense - (SC) Pine sand, trace silt - brown - moist - medium dense (estimated) cobble at 19.75 feet - (SP) Clayey silt, trace interbedded sand seems - grayish brown - wet to saturated - medium dense (ML - CL) 84 852 5 **8**/1 7 8 0 **⊗**#7 silty fine to:medium.sand, trace gravel and coarse sand -brown - saturated - medium dense to vary dense - (SO) 10 11 14 88 15 16 89 55.0 Continued on next page IS STRATIFICATION UNITS REPRESENT THE APPROXIMATY BOUNDARY LINES REFINEEN BOOL TYPES IN SITE, THE TRANSITION MAY BE GRADUAL \$15.00 NO. 94026-8 SHEET NO.

TS Con					ECT NAME posed Well Ho	1160		RCHITE		GINE	ER				
SITE LO					Louis Park,			EKT, In	<u>e.</u>	Ю		D COM	100 June		
		_	_	7	Die Falk,				_		I	1	1	1	5
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SO MANY CONSIDERATIONS ENTER INTO THE MAKING OF A GOOD WELL THAT, WHILE WE BELIEVE SLOT SIZES FURNISHED OR RECOMMENDED FROM SAND SAMPLES ARE CORRECT WE ASSUME NO RESPONSIBILITY FOR THE SUCCESSFUL OPERATION OF ANY WELL.

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80 MANY CONSIDERATIONS ENTER INTO THE MAKING OF A GOOD WELL THAT, WHILE WE BELIEVE BLOT SIZES FURNISHED OR RECOMMENDED FROM SAND SAMPLES ARE CORRECT WE ASSUME NO RESPONSIBILITY FOR THE SUCCESSFUL OPERATION OF ANY WELL.

environmental and engineering excellence

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